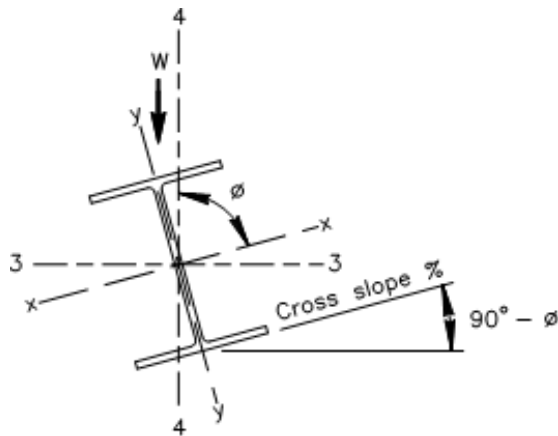


Appendix D Example 3 – Falsework Beam – Bi-Axial Bending – Canted > 2%

This example demonstrates how to calculate maximum bending stress in beams canted more than two percent. Refer to *Falsework Manual*, Section 5-4.04, *Bi-Axial Bending*.

Given Information

Span = 48 Ft	Member W 14 x 176
Cross slope = 10%	$I_{x-x} = 2140 \text{ In}^4$ $I_{y-y} = 838 \text{ In}^4$
	$D = 15.22 \text{ In}$ $b_f = 15.65 \text{ In}$



$$\phi = 90^\circ - \tan^{-1}(\text{cross slope})$$

$$= 90^\circ - \tan^{-1}\left(\frac{10}{100}\right) = 84.29^\circ$$

$$y = \frac{d}{2} = \frac{15.20 \text{ inches}}{2} = 7.60 \text{ inches}$$

$$x = \frac{b_f}{2} = \frac{15.70 \text{ inches}}{2} = 7.85 \text{ inches}$$

Uniform Load W:

Total Section:

Load A = Concrete (160 lb/ft³) + Beam (176 lb/ft) + LL (20 lb/ft²) = 1420 lb/ft

Load B = Concrete only (150 lb/ft³) = 1000 lb/ft

Bottom slab and stems:

Load C = Concrete (150 lb/ft³) = 649 lb/ft

Assume lateral bracing is adequate so that $F_b = 22,000$ psi maximum of the Standard Specifications is not exceeded.

$$\phi = 90^\circ - \tan^{-1} \frac{10}{100} = 84.29^\circ$$

Check Bending and Deflection

Check bending:

$$M = \frac{WL^2}{8} = \frac{(1420 \text{ lb/ft})(48 \text{ ft})^2}{8} = 408,960 \text{ ft-lbs} = 4,907,520 \text{ in-lbs}$$

$$f_b = 4,907,520 \left(\frac{7.60}{2140} \sin 84.29^\circ + \frac{7.85}{838} \cos 84.29^\circ \right) = 21,915 \text{ psi} < 22,000 \text{ psi}$$

allowable **OK**

Check deflections:

Check x and y deflections versus L/240 using Load B:

Load in the y-direction $W_y = 1000(\cos(90-84.29)) = 995.04 \text{ lb/ft}$

$$\Delta_y = \frac{5WL^4}{384EI_{x-x}} = \frac{5(995.04 \text{ lb/ft})(48 \text{ ft})^4 \left(1728 \frac{\text{in}^3}{\text{ft}^3}\right)}{384 (30 \times 10^6 \text{ psi})(2140 \text{ in}^4)}$$

$$= 1.85 \text{ in.} < \frac{L}{240} = \frac{(48)(12)}{240} = 2.4 \text{ inches allowable}$$

Load in the x-direction $W_x = 1000(\sin(90-84.29)) = 99.49 \text{ lb/ft}$

$$\Delta_x = \frac{5WL^4}{384EI_{y-y}} = \frac{5(99.49 \text{ lb/ft})(48 \text{ ft})^4 \left(1728 \frac{\text{in}^3}{\text{ft}^3}\right)}{384 (30 \times 10^6 \text{ psi})(838 \text{ in}^4)}$$

$$= 0.47 \text{ in.} < \frac{L}{240} = \frac{(48)(12)}{240} = 2.4 \text{ inches allowable}$$

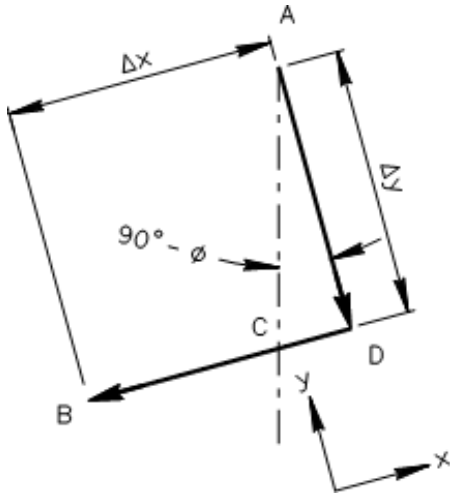
Check Δ_x versus max allowable of 1.5 inches using Load C:

Load in the x-direction $W_x = 649 (\sin (90-84.29)) = 64.57 \text{ lb/ft}$

$$\Delta_x = \frac{5WL^4}{384EI_{y-y}} = \frac{5(64.57 \text{ lb/ft})(48 \text{ ft})^4 (1728 \text{ in}^3/\text{ft}^3)}{384 (30 \times 10^6 \text{ psi})(838 \text{ in}^4)} = 0.31 \text{ in.}$$

Load in the y-direction $W_y = 649(\cos(90-84.29)) = 645.78 \text{ lb/ft}$

$$\Delta_y = \frac{5WL^4}{384EI_{x-x}} = \frac{5(645.78 \text{ Lb/Ft})(48 \text{ Ft})^4 (1728 \text{ In}^3/\text{Ft}^3)}{384 (30 \times 10^6 \text{ psi})(2140 \text{ In}^4)} = 1.20 \text{ in.}$$



Lateral deflection = CB.

$$DC = AD [\tan(90^\circ - \phi)]$$

$$= 1.20 [\tan(90^\circ - 84.29^\circ)] = 0.12 \text{ in.}$$

$$CB = DB - DC$$

$$= 0.31 - 0.12 = 0.19 < 1.5 \text{ in. } \mathbf{OK}$$